

PERFORMANCE OF RICE HUSK ASH MIXTURE AS A SUSTAINABLE CONSTRUCTION MATERIAL.

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Abstract

Finding a replacement for cement to assure sustainability is crucial as the raw materials used in making cements which are naturally occurring are depleting. The raw materials are directly or indirectly mined each year for cement manufacturing and it is time to look into the use of agriculture waste by-products in replacing cement. Rice husk ash (RHA) which has the pozzolanic properties is a way forward. An intensive study on RHA was conducted to determine its suitability. It shows that up to 30% replacement of OPC with RHA has the potential to be used as partial cement replacement (PCR), having good compressive strength performance and durability, thus have the potential of using RHA as PCR material and this can contribute to sustainable construction.

Keywords

Rice Husk Ash, Pozzolan, Compressive Strength, Permeability

1. Introduction.

In this century, the utilization of rice husk ash (RHA) as cement replacement is a new trend in concrete technology. Besides, as far as the sustainability is concerned, it will also help to solve problems otherwise encountered in disposing of the wastes. Disposal of the husks is a big problem and open heap burning is not acceptable on environmental grounds, and so the majority of husk is currently going into landfill. The disposal of rice husks create environmental problem that leads to the idea of substituting RHA for silica in cement manufactured. The content of silica in the ash is about 92-97%.

Research had shown that small amounts of inert filler have always been acceptable as cement replacements, what more if the fillers have the pozzolanic properties, in which it will not only impart technical advantages to the resulting concrete but also enable larger quantities of cement replacement to be achieved. There are many advantages in using pozzolans in concrete, and they are; improved strength and durability workability, high resistance to

chemical attack and low diffusion rate of chloride ions resulting in a higher resistance to corrosion of steel in concrete, reduced material cost, environmental benefits. Studies had showed the outstanding

technical benefit of incorporating cement replacement materials (RHA) in which it significantly improves the durability properties of concrete. These properties are difficult to achieve by the use of pure Portland cement alone.

In the present investigation, rice husk ash was blended with ordinary Portland cement at various percentages by simple replacement method and a realistic assessment of the properties has been made in addition to the mechanical properties and the results were compared with conventional Portland cement concrete.

2. Experimental Details:

2.1 Materials used

Ordinary Portland cement (OPC): Conforming to IS 8112-1989 was used for the investigation is given in Table 1.

Graded fine aggregate: Local clean river sand (fineness modulus of medium sand equal to 2.46) zone III

Graded coarse aggregates: Locally available well graded aggregates of normal size greater than 4.75 mm and less than 12 mm.

2.2 Material preparation

The rice husk was burnt in a ferro cement furnace to produce RHA. After burning and allow cooling inside the furnace for another 24 hours, the burnt ashes were taken out for grinding using a Los Angeles (LA) machine. Figure 1 shows the products obtained after the operation, i.e. rice husk ash (RHA).



Fig

ure 1: Product produced after each process

2.3 Commercial production of rice husk ash



2.4 Chemical Composition of RHA And OPC

Table 1

Chemical Composition (%)	SiO ₂	Al ₂ O	Fe ₂ O ₃	TiO ₂	MgO
OPC	15.05	2.56	4.00	0.12	1.27
RHA	96.7	1.01	.05	0.16	0.19

3. Test Conducted

- (a) Compression test,
- (b) Split tensile test,
- (c) Pull-out test,
- (d) Effective porosity,
- (e) Coefficient of water absorption,
- (f) Rapid chloride ion penetration test (RCPT)

3.1. Compressive strength

Compressive strength test was carried out in concrete cubes of size 100 · 100 · 100 mm using 1:1.5:3.0 mix with W/C ratio of 0.53. Specimens with ordinary Portland cement concrete (control) and OPC replaced by rice husk ash at 5%, 10%, 15%, 20%, 25% and 30% replacement levels were cast. During moulding, the cubes were mechanically vibrated. After 24 h, the specimens were removed from the mould and subjected to water curing for 7, 14 and 28 days. After a specified period of curing, the specimens were tested for compressive strength using compression testing machine of 2000 kN capacity at a rate of loading of 140 kN/min. The tests were carried out on triplicate specimens and the average compressive strength values were recorded.



Figure 2 compression testing machine

3.2. Split tensile test

Concrete cylinders of size 150 mm diameter and 300 mm height were cast using 1:1.50:3.0 mix with W/C ratio of 0.53. Specimens with OPC and OPC replaced by rice husk ash at 5%, 10%, 15%, 20%, 25% and 30% replacement levels were cast. During moulding, the cylinders were mechanically vibrated using a table vibrator. After 24 h, the specimens were removed from the mould and subjected to water curing for 7, 14 and 28 days. After the specified curing period was over, the concrete cylinders were subjected to split tensile test by using universal testing machine. Tests were carried out on specimens and average split tensile strength values were recorded.

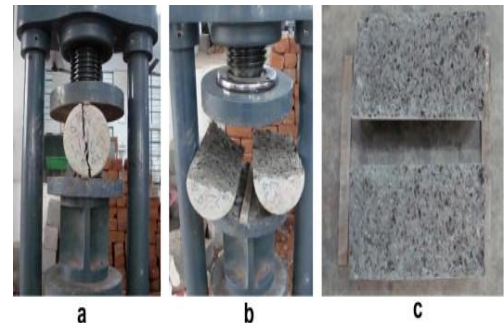


Figure 2 split testing procedure

3.3. Pull-out test

Cold twisted deformed bars of 12 mm · 450 mm long were used for steel-concrete bond strength determination. The rod was placed centrally along with helical reinforcement provided in the centre of the concrete cube of size 100 · 100 · 100 mm using a concrete mix of 1:1.50:3.0 with W/C ratio equal to 0.53. Specimens with OPC and OPC replaced by rice husk ash at 5%, 10%, 15%, 20%, 25% and 30% replacement levels were cast. The bar is projected down for a distance of about 10 mm from the bottom face of the cube as cast and projected upward from the top up to 300 mm height in order to provide an adequate length to be gripped for application of load. During casting of concrete cubes, the moulds were mechanically vibrated. The cubes were removed from the mould after 24 h and then cured for 28 days with

complete immersion in distilled water. After the curing period was over the steel-concrete bond strength was determined using Universal Testing Machine of capacity 60t. The bond strength was calculated from the load at which the slip was 0.25 mm. Tests were carried out in specimens and average bond strength values were obtained.

3.4. Effective porosity test

For determination of effective porosity and coefficient of water absorption, discs of size 83 mm diameter and 50 mm thick were cast with and without rice husk ash at 5%, 10%, 15%, 20%, 25% and 30% replacement levels and cured for 28 days in distilled water. After the curing period was over the specimens were dried in an oven at $105 \pm 5 \text{ } ^\circ\text{C}$ for 48 h in order to evaporate the moisture content present in the concrete. The effective porosity and coefficient of water absorption are calculated as follows

$$\text{Effective porosity (\%)} = \frac{(B-A)}{V} \times 100$$

Where,

A = mass of oven dried sample in air

B = saturated mass of the surface dry sample in air after immersion

V = bulk volume of the sample

3.5. Coefficient of water absorption

The same specimens used for effective porosity was used for this study also. Coefficient of water absorption is a measure of water permeability and is calculated as follows:

$$\text{Coefficient of water absorption } K_a = \frac{Q}{A} \times \frac{1}{t}$$

K_a = Coefficient of water absorption

Q = Quantity of water absorbed by the oven dried specimen in time t, t = 60 min

A = Total surface area of concrete through which water penetrates

3.6. Rapid chloride ion penetration test (RCPT)

Concrete disc of size 85 mm diameter and 50 mm thickness with and without rice husk ash were cast and allowed to cure for 28 days. After 28 days of curing the concrete specimens were subjected to RCPT test by impressing 60 V. Two halves of the specimen are sealed with PVC container of diameter 90 mm. One side of the container is filled with 3% NaCl solution (that side of the cell will be connected to the negative terminal of the power supply), the other side is filled with 0.3 N NaOH solution (which will be connected to the positive terminal of the power supply). Current is measured at

every 30 minutes up to 6 h. Chloride contamination and temperature at every 30 min were also monitored. From the results using current and time, chloride permeability is calculated in terms of Coulombs at the end of 6 h.

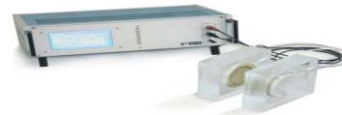


Figure 5 Rapid Chloride Ion Penetration Apparatus

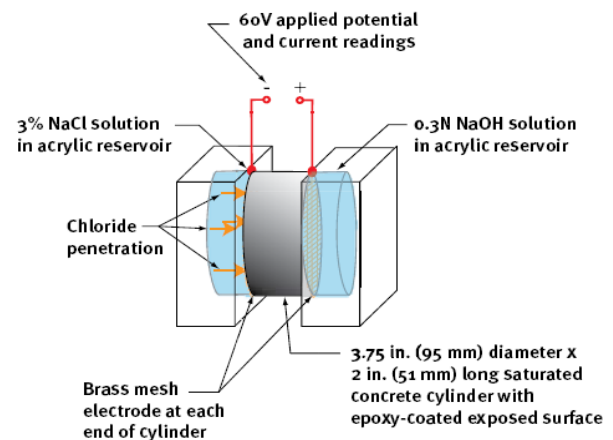


Fig. 1: Schematic of rapid chloride permeability test setup

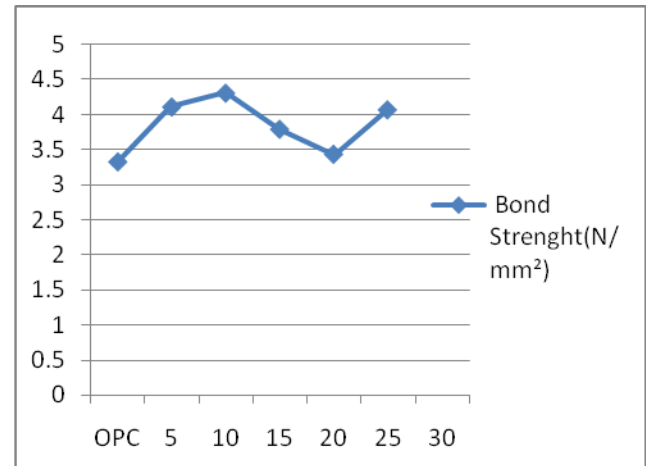
4. Results and discussion

4.1. Compressive strength

Table shows the compressive strength of rice husk ash replaced concrete after 7, 14 and 28 days of curing. From the table, it is found that the compressive strength increases with blending percentage and with age. This value is pronounced for all replacement levels. Higher concentration RHA also can be used without strength loss. After 28 days of curing also all the rice husk ash replaced concretes are showing a higher compressive strength than the control concrete. After 28 days of curing, conventional and 5% rice husk ash replaced concretes are showed equal compressive strength. Rice husk ash blended concretes showed higher compressive strength than control concretes beyond 5% replacement levels. Up to 30% replacement level of rice husk ash there is no decrease in compressive strength observed when compared to conventional OPC concrete.

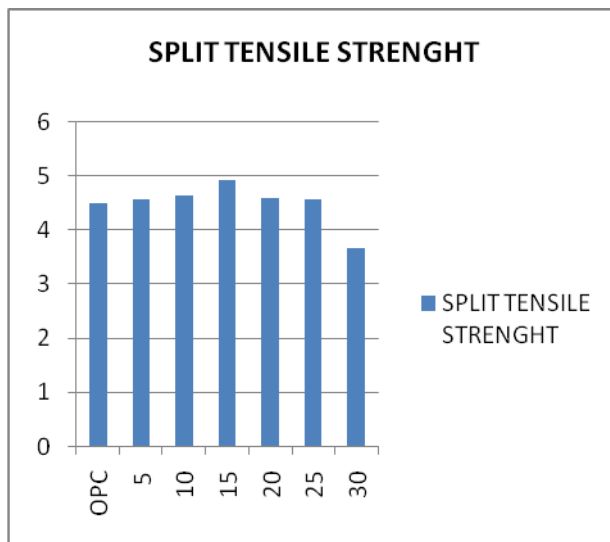
compressive strength of rice husk ash replaced concrete after, 7, 14, 28 days of curing

% Replacement	Average compressive strength(N/mm ²)		
	7 days	14 days	28 days
OPC	27.22	33.29	36.45
5	31.32	35.62	36.49
10	30.45	35.97	37.43
15	31.52	35.04	37.38
20	31.64	36.17	37.71
25	33.09	35.27	39.55
30	33.53	35.44	37.80



4.2. Split tensile strength

The split tensile strength (N/mm²) of rice husk ash blended concrete up to 30% replacement levels after 28 days curing are shown in graph 1. It can be observed from the table that, up to 25% replacement of rice husk ash the split tensile strength has not been affected. After 25% replacement level, a slight decrease in split tensile strength is observed.



4.3. Bond strength

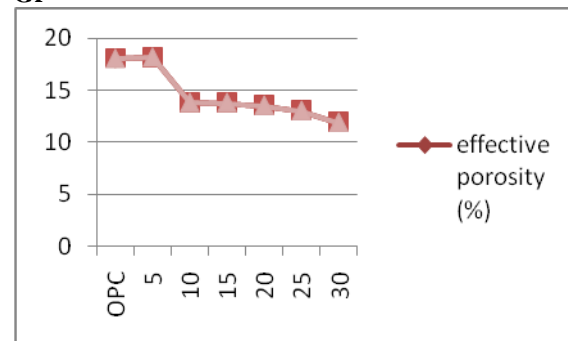
Graph 2 shows the bond strength results of OPC and rice husk ash replaced concrete after 28 days of curing. From the table it is observed that, all the rice husk replaced concretes are showing higher bond strength values than the conventional concrete. So the replacement of rice husk ash does not affect the bond strength properties.

Graph 2

4.4. Effective porosity test

Graph 3 shows the porosity values of OPC and different percentage of rice husk ash replaced concrete after 28 day curing. From the table, it is observed that the porosity values decrease as the percentage of replacement increases. It has also been reported that at early age of curing (7 and 28 days) supplementary cementitious materials are more porous than the plain cement paste and the pore size distributions are more porous, but at the later ages (90 days) this may be reversed. They have also reported that pozzolanic materials increased the porosity and reduced the pore structure. The same trend is observed in this case also. The small RHA particles improved the particle packing density of the blended cement, leading to a reduced volume of larger pores.

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4.5. Coefficient of water absorption

Table 3 indicates the coefficient of water absorption of OPC and different percentages of rice husk ash replaced concrete after 28 days curing. From the table it is observed that, the coefficient of water absorption for rice husk ash replaced concrete at all

replacement levels is found to be less when compared to control concrete.

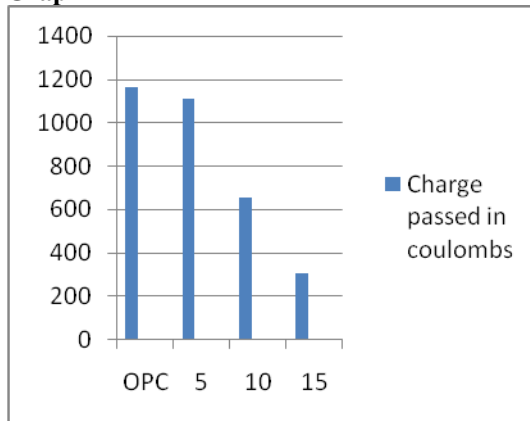
Table 3

% Replacement	Coefficient Of Water Absorption(m ² /s)
OPC	3.55×10^{-10}
5	6.75×10^{-11}
10	1.03×10^{-11}
15	1.06×10^{-11}
20	1.21×10^{-10}
25	1.45×10^{-10}
30	1.30×10^{-10}

4.6. Rapid chloride ion penetration test

Graph 4 shows the rapid chloride permeation test results of rice husk ash replaced concrete after 28 days curing. From the table it is found that as the replacement level increases the charge passed decreases. Replacement of rice husk ash drastically reduced the Coulomb values. As the replacement level increases, the chloride penetration decreases.

Graph 4



5. Conclusion

From the above investigations it is found that the incorporation of RHA up to 30% replacement level reduces the chloride penetration, decreases permeability, improves strength and corrosion resistance properties. The RHA containing concrete has more resistance to chemical attacks than OPC concrete mixture containing 25% RHA as a replacement of OPC produced the same strength as the concrete containing 100% OPC. Therefore, this concrete could be used to reduce environmental problems associated with OPC production and RHA dumping. Partial replacement of cement by RHA can result in substantial cost savings with added benefits of alleviating environmental problems related to cement production. From this

study it is concluded that the replacement level of RHA is recommended up to 25%.

References.

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